

Comparative Study on Dynamic Analysis of a Multi-Storey Frame in Zone III and Zone V

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Abstract: In most of investigation performed by specialists or structure planners are static that powers changes at such a quickened way, that it's nearly considered as consistent, succeeding no unique impacts are taken up by them. In any case, if the powers experienced by the structure is changing so that inertial powers have generous impact on the steadiness of the concerned structure, at that point such unique examination is important to assess its genuine presentation under unique excitation with the goal that expert come to think about the basic prerequisites of structure to be sheltered and practical. Former incidence of earthquake consequences in break down of building which were not predominantly contrived to be earth quakeresistant. In interpretation of this, the structure has to be premeditated with seismic confrontation. An earthquake is the outcome of an abrupt release of dynamism in the Earth's crust that crafts seismic waves. It is also known as a quake, tremor or temblor. An earthquake is instigated by a sudden slip on a fault.

Index Terms: Dynamic Analysis, Response Spectrum Method, Zone III and V, Shapes H and T.

I. INTRODUCTION

Nowadays, most of the researchers are working on enhancement of seismic resistance behavior of RC frame structures as demonstrated by research and field observations. They are likely to suffer significant damage even for moderate earthquakes. In addition to the economic loss, seismic-deficient buildings may cause injuries and casualties. The seismic engineering research community has dedicated significant efforts in developing retrofit measures to address these issues. The previous reports presents a literature review of experimental and numerical investigations on the seismic strengthening of reinforced concrete (RC) buildings, focusing on the use of steel bracings, infills and shear walls. Strengthening is a promising strategy, as nowadays reduced drifts and non-structural damage are becoming important performance requirements. It also introduces the available retrofit measures and their possible effects on the local and global response of a building. In addition to the technical aspects, socio-economic requirements affect the choice of the measures to implement, as illustrated in a cost-benefit case study of a real RC building. Two techniques, namely incremental retrofit and selective weakening, that have not been extensively applied and verified are also presented. Practical applications range from new walls constructed externally to the frame, to infilling of bays with reinforced concrete, and the most technologically advanced hybrid walls, i.e. rocking walls with energy-dissipating devices.

Earthquakes are one of the most feared natural phenomena that are relatively unexpected which results in the loss of life. In past occurrence of earthquake results in collapse of building which were not particularly engineered to be earthquake resistant. In view of this the structure has to be designed with seismic resistance. An earthquake is the result of a sudden release of energy in the Earth's crust that creates seismic waves. It is also known as a quake, tremor or temblor. An earthquake is caused by a sudden slip on a fault. The tectonic plates are always slowly moving, but they get stuck at their edges due to friction. When the stress on the edge overcomes the friction, there is an earthquake that releases energy in waves that travel through the earth's crust and causes shaking.

Because of improper plan of the structure without seismic opposition numerous structures have crumpled and lives have lost. Numerous investigations like Base Isolation, Damping Devices, shear divider and Bracing Systems have been done to defeated this need so as to ensure the structure and lives which does not improve the exhibition of the structure. Among these strategies, shear divider and bracings has been decided for this investigation. Shear dividers ought to be situated on each dimension of the structure including the creep space. To shape a compelling box structure, equivalent length shear dividers ought to be set symmetrically on every one of the four outside dividers of the structure. Shear dividers ought to be added to the structure inside when the Exterior dividers can't give adequate quality and

solidness or when the admissible range width proportion for the floor or rooftop. For subfloors with customary askew sheathing, the range width proportion is 3:1. This implies a 25-foot wide structure with this subfloor won't require inside shear dividers until its length surpasses 75 feet except if the quality or solidness of the outside shear dividers are lacking.

Shear dividers are vertical components of the flat power opposing framework. They are commonly wood outline stud dividers secured with an auxiliary sheathing material like compressed wood. At the point when the sheathing is appropriately secured to the stud divider confining, the shear divider can oppose powers coordinated along the length of the divider. At the point when shear dividers are structured and built appropriately, they will have the quality and firmness to oppose the level powers. A propping framework serves to balance out the primary braces amid development, to contribute the circulation of burden impacts and to control the pressure spines or harmonies where they would some way or another be allowed to clasp horizontally. Propping gives at least one of the accompanying capacities:

1. Control clasping of the principle pillars
2. Burden conveyance
3. Dimensional control.

Since propping associates pillars, it tends to be utilized to disperse the vertical bowing impacts between the principle shafts and to guarantee that horizontal impacts, for example, wind stacking and crash stacking are shared between every one of the bars. This sharing is especially significant at lines of help, where the impacts of the horizontal burdens are frequently opposed at one fixed or guided bearing.

The plan approach received in the Indian Code IS 1893(Part I): 2002 'Criteria for Earthquake Resistant Design Of Structures' is to guarantee that structures have in any event a base solidarity to withstand minor seismic tremor happening oftentimes, without harm; oppose moderate quakes without critical auxiliary harm however some non-basic harm may happen; and points that structures withstand serious tremor without breakdown. Structures need reasonable seismic tremor safe highlights to securely oppose huge parallel powers that are forced on them amid continuous quakes. Normal structures for houses are generally worked to securely convey their very own

loads. Low sidelong loads brought about by wind and subsequently, perform ineffectively under huge parallel powers brought about by even moderate size seismic tremor. These horizontal powers can deliver the basic worries in a structure, set up unfortunate vibrations and, furthermore, cause sidelong influence of structure, which could achieve a phase of distress to the inhabitants.

Shear divider is a standout amongst the most generally utilized horizontal burden opposing component in elevated structure. Shear divider (SW) has high in plane solidness and quality which can be utilized at the same time to oppose huge even burden and bolster gravity load. The extent of present work is to think about and research the adequacy of RC shear divider in medium ascent building. Fortified solid shear dividers are utilized in exposed edge working to oppose parallel power because of wind and tremors. They are typically given between section lines, in stair wells, lift wells, in shafts. Shear divider give sidelong burden opposing by exchanging the breeze or quake burden to establishment. Plus, they bestow sidelong firmness to the framework and furthermore convey gravity loads. Be that as it may, exposed casing with shear divider still become financially ugly. On the off chance that the basic specialists consider property the non-basic component in basic structure alongside different components like shear divider gives better outcomes.

The best and useful technique for upgrading the seismic opposition is to expand the vitality ingestion limit of structures by joining supporting components in the casing. The supported casing can ingest a more prominent level of vitality applied by tremors. Propping individuals are generally utilized in steel structures to decrease parallel uprooting and scatter vitality amid solid ground movements. This idea stretched out to solid edges. The different angles, for example, size and state of structure, area of shear divider and propping in structure, circulation of mass, appropriation of firmness incredibly influence the practices of structures. Supporting framework improves the seismic presentation of the edge by expanding its sidelong firmness and limit. To the expansion of supporting framework burden could be exchanged out of the edge and into the props, by passing the feeble sections. The solidness included by the propping framework is kept up nearly up to the pinnacle quality. Solidness is especially significant at functionality state, where distortions are restricted to forestall harm.

II. RESEARCH GAP

In the past studies specified by various authors, shear wall at different locations and different bracings were utilized in frames so as to study their seismic effects. Now, in the present study, it has been decided to model frames with combinations of shear wall and bracings at different locations and in different zones. Linear dynamic behavior of moment resisting frames was studied with the different combinations, so as a result to make a comparative relation and to have a precise position of shear wall and steel bracings. In the present study, research gap is covered with the efforts made to cover the wide range of comparative study cases of combination of bracings and steel bracings.

III. OBJECTIVES OF THE STUDY

- I. To compare symmetrical and unsymmetrical building frames subjected to seismic excitations.
- II. To study the inter story drift, base shear and displacement at nodes for H-type and T-type frames consisting of bracings.
- III. To study the inter story drift, base shear and displacement at nodes for H-type and T-type frames consisting of shear wall at different locations.
- IV.

V. RESEARCH METHODOLOGY

In the current study, importance was through on the analysis of H-type and T-type frames using Response Spectrum Method. A (G+13) building was modeled for the study. Different building models were prepared consisting of shear wall and composite bracings at different locations so as to make a comparative study for precise solutions. By doing so, effective location of shear wall and bracing can be analyzed. The parameters on which the comparative study was made are: Base shear, Story Drift and Displacement at nodes.

Table 1: Specifications of the building

Specifications	Data
Story Height	3.0m
Bays along X direction	3
Bays along Z direction	3
Bay Length along X direction	5m
Bay Length along Z direction	5m
Grade of Concrete	M 40
Columns	0.45m x 0.45m
Longitudinal Beams	0.45m x 0.25m
Transverse Beams	0.35m x 0.25m
Slab Thickness	0.15m
Unit Weight of Concrete	25 kN/m ³
L.L.	4.0 kN/m ³

Building Type	General Building (I=1)
Type of soil	Medium Soil
Response Reduction factor	5
Damping ratio	5%
Bracing	ISHB250
Zone	IV

4.1. Following are Various Study Cases Enrolled in Study:
Case 1: RC H-type Frame Structure

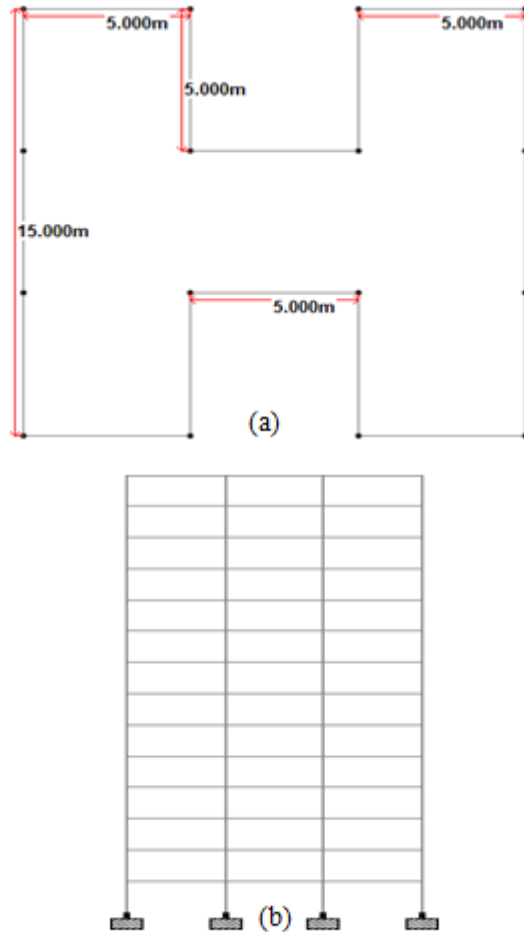


Fig. 1: Plan and Elevation of H Shape Bare Frame

Case 2: RC T Shape Frame Structure

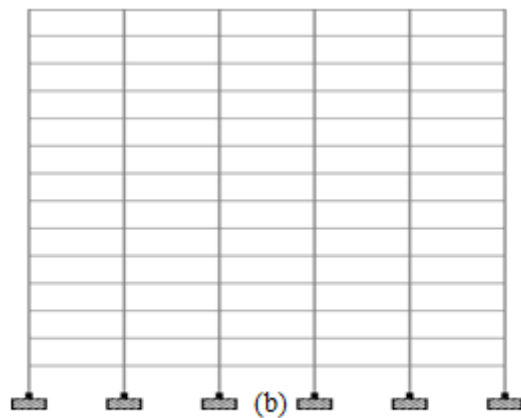
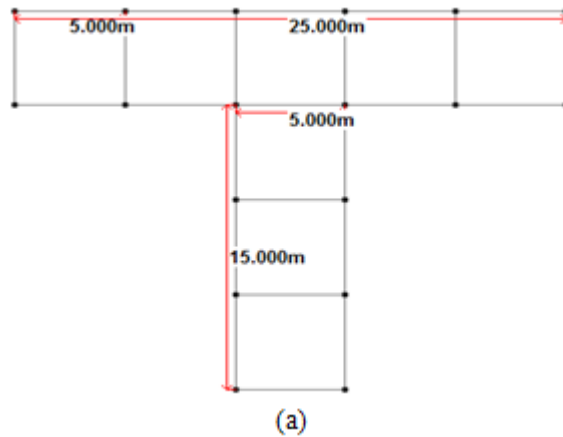


Fig. 2: Plan and Elevation of RC T Shape Frame

V. RESULTS AND DISCUSSIONS

Deals with the results obtained from the H type and T type frames consisting of different configurations, when they were subjected to seismic excitations. The results then obtained from different frames were compared on the basis of base shear, storey drift and displacement.

Table 2: Case 1 RC Frame Structure

Parameters	Displacement (mm)	Storey Drift (mm)	Base Shear (kN)
H type Frame	310.857	0.007548	3027.54
T type Frame	357.451	0.011578	3347.52

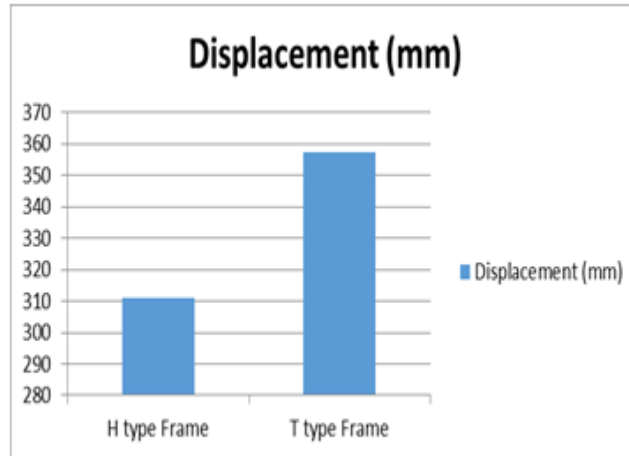


Fig. 3 Graph Representing Displacement Values for H-type and T-type Frames

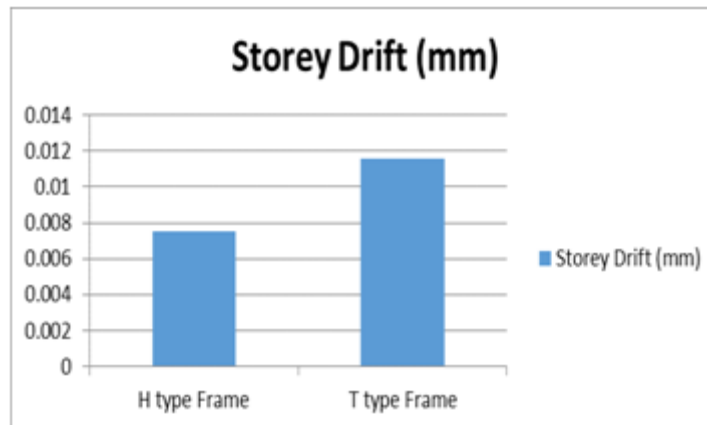


Fig. 4 Graph Representing Storey Drift for H-type and T-type Frames

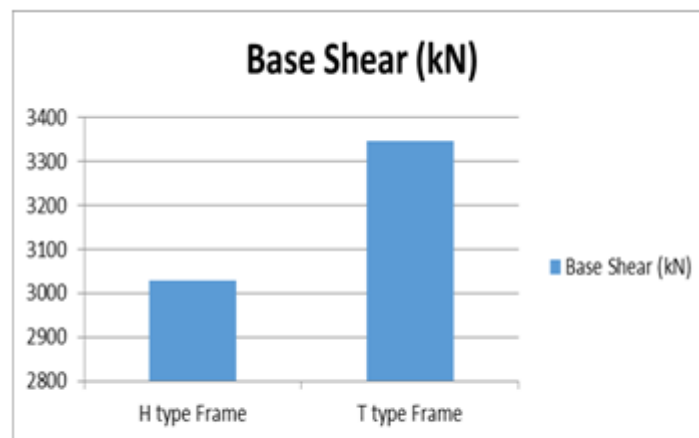


Fig. 5 Graph Representing Base Shear for H-type and T-type Frames

Case 2: RC Frame Structure with Combination of Shear Wall and Composite Bracing Systems

Table 3: Representing Displacement Values for H-type Systems for Various Composite Bracing and RCShear Wall Systems

Types	Displacement (mm)	
	Zone III	Zone V
K - Core	112.569	193.854
V - Core	110.215	185.325

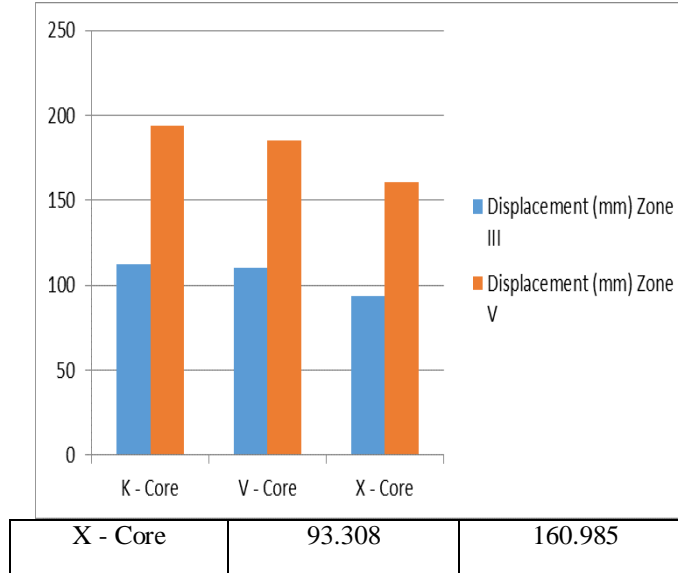


Fig. 6 Graph Representing Displacement Values for H-type Systems for Various Composite Bracing and RCShear Wall Systems

VI CONCLUSION

Case 1: RC Frame Structure: Bare RC frame structures, H-type and T-type in plan were analyzed for dynamic analysis in Staad Pro V8i. It was found that there is great significance in comparing the results of them on the basis of base shear, storey drift and displacement at nodes. The displacement was calculated for H-type frame as 310.857mm, whereas for T-type frame as 357.451mm. The storey drift was calculated for H-type frame as 0.007548mm, whereas for T-type frame as 0.011578mm. The base shear was calculated for H-type frame as 3027.54 kN, whereas for T-type frame as 3347.52kN.

Case 2: RC Frame Structure with Different Shear Wall System and Composite Bracings: For this particular case, bracings and shear wall were united together in H type and T type frames so as to make more informative study and to have more accurate results for precise comparison. For H shape frames, several combinations were made, but among all united X type bracing and corner shear wall had shown best results in displacement as 63.458mm in zone III and 139.962mm in zone V. Similarly in T shaped frames, diamond shaped bracing with provision of shear wall at edges had shown least displacement as 75.145mm.

VII. FUTURE SCOPE

In this research, attention was made on linear dynamic analysis for seismic excitation. But, non-linear dynamic analysis can also be performed on the frame structure for precise evaluation of results. So, non-linear time history analysis can also be performed. Also there can be option for wind analysis.

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